



September 12, 2008

TO: PARTIES INTERESTED IN EVALUATION REPORTS ON BELLED SEGMENTED PIPE FOUNDATION SYSTEMS AND DEVICES

SUBJECT: Proposed Acceptance Criteria for Belled Segmented Pipe Foundation Systems and Devices, Subject AC406-1008-R1, (MO/BG)

Hearing Information:

Wednesday, October 22, 2008
8:00 a.m.

Holiday Inn, Albany on Wolf Road
205 Wolf Road
Albany, New York 12205
(800) 465-4329

Dear Madam or Sir:

The subject proposed new acceptance criteria will be on the agenda for the Evaluation Committee meeting noted above. The criteria is a proprietary product criteria covering belled segmented steel pipe foundation systems and devices. The foundation system consists of segmented steel pipe sections, a steel bracket connection device attached to the structure, a coupler device for joining steel pipe segments, and a steel bullet assembly that is advanced into the soil by pressure and expands to an increased bearing area at the final depth.

The proposed criteria is based on the ICC-ES Acceptance Criteria for Helical Foundation Systems and Devices (AC358), approved June 2007. Sections where the proposed criteria differs from AC358 are noted as follows: The belled segmented pipe foundation system is described in Section 1.4.1 of the criteria; design, testing and performance for the bracket assembly are covered in Sections 3.2, 3.10 and 4.1; segmented pipe sections are covered in Sections 3.3, 3.11 and 4.2; the bullet assembly is covered in Sections 3.4, 3.12 and 4.3; and soil bearing capacity of the foundation system is covered in Sections 3.5, 3.13 and 4.4.

So the committee can consider the requirements for the proprietary product alone, AC406 is offered as a separate document; however, it is staff's intent to consolidate this criteria, if approved, with AC358. Product definitions, testing and performance requirements that are unique to each foundation system will be located in appropriate appendices. General requirements covering both foundation systems will be in the body of AC358.

Attached to this cover letter are photos of the components of the proprietary system.

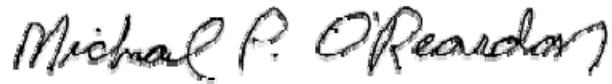
You are cordially invited to submit written comments on agenda items, or to attend the Evaluation Committee hearing and present verbal comments. If you wish to contribute to the hearing, please note the following:

1. Written comments that are received by the Los Angeles business/regional office by **October 7, 2008**, will be forwarded to the committee prior to the hearing, and will be posted on the ICC-ES web site shortly after the comment deadline.
2. Written comments received up to ten days before the meeting, and staff memos responding to comments, will be posted to the web site on **October 16, 2008**.
3. ICC-ES is no longer providing printed copies at the meeting of proposed acceptance criteria, staff memos or public comments. These documents will be available on a limited number of CDs at the meeting, for uploading to computers; and ICC-ES will make arrangements with the hotel business center to have hard copies available for photocopying.
4. Written comments that miss the deadline noted in item (1), above, will only be available at the meeting if you provide 35 copies, collated, stapled, and three-hole punched, either at the meeting itself or to the Los Angeles business/regional office by **October 16, 2008**.
5. If you plan to speak for more than 15 minutes, or offer a visual presentation lasting longer, you should notify ICC-ES staff as far as possible in advance. There will be a computer, projector, and screen available at the meeting for anyone wishing to make a visual presentation, and presentations in most cases will need to be in PowerPoint format. Also, ICC-ES will need to be provided with your presentation at least a half-hour before the start of the relevant meeting session (morning or afternoon) on either a CD or a flash card.
6. If you have any special needs related to a presentation, you should contact ICC-ES staff well in advance of the meeting.
7. Any visual aids for viewing at committee meetings (charts, overhead transparencies, slides, videos, electronic presentations, etc.) will be permitted only if a copy is provided to ICC-ES, before the presentation, in a medium that can be retained with other records of the meeting.
8. Any materials submitted for committee consideration are considered nonconfidential and available for public discussion, as noted in Section 2.7 of the ICC-ES Rules of Procedure for the Evaluation Committee.
9. Prior to the meeting, you should refrain from trying to communicate directly with committee members about agenda items, either verbally or in writing. Committee members reserve the right to refuse such communications.

Your cooperation with these guidelines is much appreciated, as is your interest in the deliberations of the Evaluation Committee. If you have any questions, please contact the

undersigned at (800) 423-6587, extension 5685. You may also reach us by e-mail at es@icc-es.org.

Yours very truly,

A handwritten signature in black ink that reads "Michael P. O'Reardon". The signature is written in a cursive, slightly slanted style.

Michael P. O'Reardon, P.E.
Senior Staff Engineer

MPO/raf

Enclosures

cc: Evaluation Committee









ICC EVALUATION SERVICE, INC., RULES OF PROCEDURE FOR THE EVALUATION COMMITTEE

1.0 PURPOSE

The purpose of the Evaluation Committee is to monitor the work of ICC-ES, in issuing evaluation reports; to evaluate and approve acceptance criteria on which evaluation reports may be based; and to sponsor related changes in the applicable codes.

2.0 MEETINGS

2.1 The Evaluation Committee shall schedule meetings that are open to the public in discharging its duties under Section 1, subject to Section 3.

2.2 All scheduled meetings shall be publicly announced.

2.3 Two-thirds ($\frac{2}{3}$) of the voting Evaluation Committee members shall constitute a quorum. A majority vote of members present is required on any action.

2.4 In the absence of the nonvoting chairman-moderator, Evaluation Committee members present shall elect an alternate chairman from the committee for that meeting. The alternate chairman shall be counted as a voting committee member for purposes of maintaining a committee quorum and to cast a tie-breaking vote of the committee.

2.5 Minutes of the meetings shall be kept.

2.6 An electronic audio record of meetings shall be made by ICC-ES; no other audio, video, electronic or stenographic recordings of the meetings will be permitted. Visual aids (including, but not limited to, charts, overhead transparencies, slides, videos, or presentation software) viewed at meetings shall be permitted only if the presenter provides ICC-ES before presentation with a copy of the visual aid in a medium which can be retained by ICC-ES with its record of the meeting and which can also be provided to interested parties requesting a copy. A copy of the ICC-ES recording of the meeting and such visual aids, if any, will be available to interested parties upon written request made to ICC-ES together with a payment as required by ICC-ES to cover costs of preparation and duplication of the copy. These materials will be available beginning five days after the conclusion of the meeting but will no longer be available after one year from the conclusion of the meeting.

2.7 Parties interested in the deliberations of the committee should refrain from communicating, whether in writing or verbally, with committee members regarding agenda items. All written communications and submissions regarding agenda items should be delivered to ICC-ES. All such written communications and submissions shall be considered nonconfidential and available for discussion in open session of an Evaluation Committee meeting, and shall be delivered at least ten days before the scheduled Evaluation Committee meeting if they are to be forwarded to the committee. Materials delivered to ICC-ES at least ten

days before the scheduled meeting will be posted on the ICC-ES web site (www.icc-es.org) prior to the meeting. After this time, parties wishing to submit materials for consideration by the Evaluation Committee must deliver a sufficient number of copies as directed by ICC-ES. Consideration of materials not received by ICC-ES at least ten days before the meeting is at the discretion of the Evaluation Committee. Following the meeting, ICC-ES will make all materials considered by the Evaluation Committee available on the web site for a maximum period of one year following the meeting. The committee reserves the right to refuse recognition of communications which do not comply with the provisions of this section.

3.0 CLOSED SESSIONS

Evaluation Committee meetings shall be open except that the chairman may call for a closed session to seek advice of counsel.

4.0 ACCEPTANCE CRITERIA

4.1 Acceptance criteria are established by the committee to provide a basis for issuing ICC-ES evaluation reports on products and systems under codes referenced in Section 2.0 of the Rules of Procedure for Evaluation Reports. They also clarify conditions of acceptance for products and systems specifically regulated by the codes.

Acceptance criteria may involve a product, material, method of construction, or service. Consideration of any acceptance criteria must be in conjunction with a current and valid application for an ICC-ES evaluation report, an existing ICC-ES evaluation report, or as otherwise determined by the Evaluation Committee.

4.2 Procedure:

4.2.1 Proposed acceptance criteria shall be developed by the ICC-ES staff and discussed in open session with the Evaluation Committee during a scheduled meeting, except as permitted in Section 5.0 of these rules.

4.2.2 Proposed acceptance criteria shall be available to interested parties at least 30 days before discussion at the committee meeting.

4.2.3 The committee shall be informed of all pertinent written communications received by ICC-ES.

4.2.4 Attendees at Evaluation Committee meetings shall have the opportunity to speak on acceptance criteria listed on the meeting agenda, to provide information to committee members.

4.3 Approval of acceptance criteria shall be as specified in Section 2.3 of these rules.

4.4 Actions of the Evaluation Committee may be

appealed in accordance with the ICC-ES Rules of Procedure for Appeal of Acceptance Criteria or the ICC-ES Rules of Procedure for Appeals of Evaluation Committee Technical Decisions.

5.0 COMMITTEE BALLOTING FOR ACCEPTANCE CRITERIA

5.1 Acceptance criteria may be issued without a public hearing following a 30-day public comment period and a majority vote for approval by the Evaluation Committee when, in the opinion of ICC-ES staff, one or more of the following conditions have been met:

1. The subject is nonstructural, does not involve life safety, and is addressed in nationally recognized standards or generally accepted industry standards.
2. The subject is a revision to an existing acceptance criteria that requires a formal action by the Evaluation Committee, and public comments raised were resolved by staff with commenters fully informed.
3. Other acceptance criteria and/or the code provide precedence for the revised criteria.

5.2 Negative votes must be based upon one or more of the following, for the ballots to be considered valid and require resolution:

- a. *Lack of clarity:* There is insufficient explanation of the scope of the acceptance criteria or insufficient description of the intended use of the product or system; or the acceptance criteria is so unclear as to be unacceptable. (The areas where greater clarity is required must be specifically identified.)
- b. *Insufficiency:* The criteria is insufficient for proper evaluation of the product or system. (The provisions of the criteria that are in question must be specifically identified.)
- c. *The subject of the acceptance criteria is not within the scope of the applicable codes:* A report issued by ICC-ES is intended to provide a basis for approval under the codes. If the subject of the acceptance criteria is not regulated by the codes, there is no basis for issuing a report, or a criteria. (Specifics must be provided concerning the inapplicability of the code.)

d. *The subject of the acceptance criteria needs to be discussed in a public hearings.* The committee member requests additional input from other committee members, staff or industry.

5.3 An Evaluation Committee member, in voting on an acceptance criteria, may only cast the following ballots:

- Approved
- Approved with Comments
- Negative: Do Not Proceed

6.0 COMMITTEE COMMUNICATION

Direct communication between committee members, and between committee members and an applicant or concerned party, with regard to the processing of a particular acceptance criteria or evaluation report shall take place only in a public hearing of the Evaluation Committee. Accordingly:

6.1 Committee members receiving an electronic ballot should respond only to the sender (staff). Committee members who wish to discuss a particular matter with other committee members, before reaching a decision, should ballot accordingly and bring the matter to the attention of ICC-ES staff, so the issue can be placed on the agenda of a future committee meeting.

6.2 Committee members who are contacted by an applicant or concerned party on a particular matter that will be brought to the committee will refrain from private communication and will encourage the applicant or concerned party to forward their concerns through the ICC-ES staff in writing, and/or make their concerns known by addressing the committee at a public hearing, so that their concerns can receive the attention of all committee members. ■

Effective March 18, 2008



PROPOSED ACCEPTANCE CRITERIA FOR BELLED SEGMENTED PIPE FOUNDATION SYSTEMS AND DEVICES

AC406

Proposed September 2008

PREFACE

Evaluation reports issued by ICC Evaluation Service, Inc. (ICC-ES), are based upon performance features of the International family of codes and other widely adopted code families, including the Uniform Codes, the BOCA National Codes, and the SBCCI Standard Codes. Section 104.11 of the International Building Code® reads as follows:

The provisions of this code are not intended to prevent the installation of any materials or to prohibit any design or method of construction not specifically prescribed by this code, provided that any such alternative has been approved. An alternative material, design or method of construction shall be approved where the building official finds that the proposed design is satisfactory and complies with the intent of the provisions of this code, and that the material, method or work offered is, for the purpose intended, at least the equivalent of that prescribed in this code in quality, strength, effectiveness, fire resistance, durability and safety.

Similar provisions are contained in the Uniform Codes, the National Codes, and the Standard Codes.

ICC-ES may consider alternate criteria, provided the report applicant submits valid data demonstrating that the alternate criteria are at least equivalent to the criteria proposed in this document, and otherwise meet the applicable performance requirements of the codes. Notwithstanding that a product, material, or type or method of construction meets the requirements of the criteria proposed in this document, or that it can be demonstrated that valid alternate criteria are equivalent to the criteria in this document and otherwise meet the applicable performance requirements of the codes, ICC-ES retains the right to refuse to issue or renew an evaluation report, if the product, material, or type or method of construction is such that either unusual care with its installation or use must be exercised for satisfactory performance, or malfunctioning is apt to cause unreasonable property damage or personal injury or sickness relative to the benefits to be achieved by the use of the product, material, or type or method of construction.

Acceptance criteria are developed for use solely for purposes of issuing ICC-ES evaluation reports.

PROPOSED ACCEPTANCE CRITERIA FOR BELLED SEGMENTED PIPE FOUNDATION SYSTEMS AND DEVICES

1 1.0 INTRODUCTION

2 **1.1 Purpose:** The purpose of this acceptance criteria is to establish requirements for
3 belled segmented pipe foundation systems and belled segmented pipe foundation
4 devices to be recognized in ICC Evaluation Service, Inc. (ICC-ES), evaluation reports
5 under the 2006 *International Building Code*[®] (IBC) and the 1997 *Uniform Building Code*
6 (UBC). Bases for recognition are IBC Section 104.11 and UBC Section 104.2.8.

7 Applicable code sections are IBC Sections 1801 (Foundation and Soils Investigations),
8 1802.2.4 (Pile and pier foundations), 1808 (Pier and Pile Foundations), 1809 (Driven
9 Pile Foundations), 1809.3 (Structural Steel Piles), and 1810.8 (Micopiles).

10 The reason for the development of this acceptance criteria is to supplement general
11 requirements for pile foundations in the IBC and UBC to permit evaluation of belled
12 segmented pipe foundation systems and devices.

13 **1.2 Scope:** This criteria provides methods to establish the allowable load and
14 deformation capacities of belled segmented pipe foundation systems and devices
15 used to resist axial compression, axial tension or lateral loads. This criteria applies to
16 belled segmented pipe foundation systems and devices as defined in Section 1.4, and
17 includes provisions for determining soil capacity.

18 This criteria is limited to belled segmented pipe foundation systems and devices used
19 under the following conditions:

20 **1.2.1** Support of structures in IBC Seismic Design Categories A, B, or C, or UBC

21 Seismic Zones 0, 1 or 2, only.

22 **1.2.2** Exposure conditions to soil that are not indicative of potential pile deterioration
23 or corrosion situations as defined by the following: (1) soil resistivity less than 1,000
24 ohm-cm; (2) soil pH less than 5.5; (3) soils with high organic content; (4) soil sulfate
25 concentrations greater than 1,000 ppm; (5) soils located in landfills, or (6) soil containing
26 mine waste.

27 **1.2.3** Belled segmented pipe devices manufactured from carbon steel, with optional
28 zinc or powder coatings.

29 **1.3 Codes and Referenced Standards:** Where standards are referenced in this
30 criteria, these standards shall be applied consistently with the code (IBC and/or UBC)
31 upon which compliance is based in accordance with Table 1.

32 **1.3.1** 2006 *International Building Code*[®] (IBC), International Code Council.

33 **1.3.2** 1997 *Uniform Building Code* (UBC).

34 **1.3.3** ICC-ES Acceptance Criteria for Corrosion Protection of Steel Foundation
35 Systems Using Polymer (EAA) Coatings (AC228).

36 **1.3.4** ACI 318-05, Building Code Requirements for Structural Concrete, American
37 Concrete Institute.

38 **1.3.5** ANSI/AISC 360-05, Specification for Structural Steel Buildings, American
39 Institute of Steel Construction.

40 **1.3.6** ANSI/AISC 341-05, Seismic Provisions for Structural Steel Buildings,
41 including Supplement No. 1 dated 2006, American Institute of Steel

42 Construction.

43 **1.3.7** ANSI/ASME B18.2.6-96, Fasteners for Use in Structural Applications, American
44 Society of Mechanical Engineers

45 **1.3.8** ANSI/AWS D1.1/D1.1M, Structural Welding Code—Steel (AWS D1. 1 /D1 .1
46 M), American Welding Society.

47 **1.3.9** ASTM International:

48 **1.3.9.1** ASTM A 36/A36M-04a, Standard Specification for Carbon Structural Steel.

49 **1.3.9.2** ASTM A 53/A53M-07, Standard Specification for Pipe, Steel, Black and
50 Hot-Dipped, Zinc-Coated, Welded and Seamless.

51 **1.3.9.3** ASTM A 500/A500M-07, Standard Specification for Cold-Formed Welded
52 and Seamless Carbon Steel Structural Tubing in Rounds and Shapes.

53 **1.3.9.4** ASTM A 501-07, Standard Specification for Hot-Formed Welded and
54 Seamless Carbon Steel Structural Tubing.

55 **1.3.9.5** ASTM A 618/A618M-04, Standard Specification for Hot-Formed Welded
56 and Seamless High-Strength Low-Alloy Structural Tubing.

57 **1.3.9.6** ASTM A 847/A847M-05, Standard Specification for Cold-Formed Welded
58 and Seamless High-Strength, Low-Alloy Structural Tubing with Improved Atmospheric
59 Corrosion Resistance.

60 **1.3.9.7** ASTM A 123/A123M-02, Standard Specification for Zinc (Hot-Dip
61 Galvanized) Coatings on Iron and Steel Products.

62 **1.3.9.8** ASTM A 153/A153M-05, Standard Specification for Zinc Coating (Hot-Dip)

63 on Iron and Steel Hardware.

64 **1.3.9.9** ASTM B 633-07, Standard Specification for Electrodeposited Coatings of
65 Zinc on Iron and Steel.

66 **1.3.9.10** ASTM B 695-04, Standard Specification for Coatings of Zinc Mechanically
67 Deposited on Iron and Steel.

68 **1.3.9.11** ASTM C 31/C31M-98, Standard Practice for Making and Curing Concrete
69 Test Specimens in the Field.

70 **1.3.9.12** ASTM C 39/C39M-03, Standard Test Method for Compressive Strength of
71 Cylindrical Concrete Specimens.

72 **1.3.9.13** ASTM C 109/C109M-05, Standard Test Method for Compressive Strength
73 of Hydraulic Cement Mortars (Using 2-in or (50 mm) Cube Specimens.

74 **1.3.9.14** ASTM C 191-07, Standard Test Method for Time of Setting of Hydraulic
75 Cement by Vicat Needle.

76 **1.3.9.15** ASTM D 1143-81 (1994)e1 , Standard Test Method for Piles Under Static
77 Axial Compressive Load.

78 **1.3.9.16** ASTM D 1586-99, Standard Test Method for Penetration Test and Split-
79 Barrel Sampling of Soils.

80 **1.3.9.17** ASTM D 3689-90 (1995), Standard Test Method for Individual Piles Under
81 Static Axial Tensile Load.

82 **1.3.9.18** ASTM D 3966-90 (1995), Standard Test Method for Piles Under Lateral
83 Loads.

84 **1.4 Definitions:** Terminology herein is based on the Glossary of the AISC and the
85 following definitions:

86 **1.4.1 Belled Segmented Pipe Foundation System:** A steel foundation with
87 factory-manufactured steel devices designed to resist axial compression, axial
88 tension, and/or lateral loads from structures, consisting of a bullet assembly with one
89 or more segmented pipe sections, couplers, and a bracket. The bullet assembly is
90 advanced into the ground by application of pressure against the existing structure
91 through the bracket. Coupler and segmented pipe sections are added and advanced
92 into the ground until resistance is reached suitable to activate expansion of the bullet
93 assembly to provide a belled section for additional bearing area. The expanded bullet
94 assembly is then stabilized against collapse of the belled shape in preparation for
95 application of structure loads. Structure loads are introduced into the foundation system
96 by jacking against the segmented pipe foundation. After the structure loads have been
97 applied, the segmented pipe foundation is filled with concrete for stabilization and
98 additional capacity.

99 **1.4.2 Belled Segmented Pipe Foundation Device:** For purposes of this criteria, a
100 belled segmented pipe foundation device is any part or component of a belled
101 segmented pipe foundation system.

102 **1.4.3 Bracket:** A steel connection device used to connect the segmented pipe
103 sections to the structure.

104 **1.4.4 Coupler:** A device for joining the segmented pipe sections.

105 **1.4.5 Bullet Assembly:** An assembly of steel components capable of advancing

106 through the soil with a means of expanding to an increased bearing area at the final
107 depth.

108 **1.4.6 Lateral Resistance:** Capacity of a belled segmented pipe foundation
109 system or device to resist forces acting in a direction that is perpendicular to the
110 longitudinal direction of the segmented pipe sections.

111 **1.4.7 Conventional Design:** Methods for determining design capacities of the belled
112 segmented pipe foundation system that are prescribed by and strictly in accordance with
113 standards and codes referenced in Section 1.3.

114 **1.4.8 Special Analysis:** Methods for determining design capacities of the belled
115 segmented pipe foundation system that incorporate finite element modeling, discrete
116 element modeling, strain compatibility, or other conventional analytical/numerical
117 techniques. Computer software developed for the analysis of laterally loaded piles,
118 which incorporate methods of analysis considering the nonlinear interaction of the
119 segmented pipe sections with soil, is an example of special analysis.

120 **2.0 BASIC INFORMATION**

121 **2.1 General:** The following information shall be submitted with ICC-ES evaluation report
122 applications:

123 **2.1.1 Summary Document:** A tabulated list of the belled segmented pipe
124 foundation systems, devices, and combinations thereof to be included in the ICC-ES
125 evaluation report, along with proposed structural capacities. All systems and devices shall
126 be clearly identified in the documentation with distinct product names and/or product
127 numbering.

128 **2.1.2 Product Description:** Belled segmented pipe foundation devices shall be
129 manufactured from carbon steel, with optional zinc or powder coatings. Complete
130 information shall be submitted pertaining to the belled segmented pipe foundation
131 systems or devices, including material specifications and drawings showing all
132 dimensions and tolerances, and the manufacturing processes. All materials, welding
133 processes and manufacturing procedures used in belled segmented pipe foundation
134 systems and devices shall be specified and described in quality documentation
135 complying with Section 5.2. All material specifications shall comply with ASTM, ACI,
136 AISC, UBC, or IBC requirements. Material composition, grade, and sizes of bolts and
137 fasteners shall be based on criteria in AISC, ASTM, or ANSI requirements.

138 **2.1.3 Installation Instructions:** Procedures and details regarding belled segmented
139 pipe foundation system or device installation, including product-specific requirements,
140 exclusions, limitations, and inspection requirements, as applicable.

141 **2.1.4 Packaging and Identification:** A description of the method of packaging and
142 field identification of each belled segmented pipe foundation system device.
143 Identification provisions shall include the manufacturer's name and address, product name
144 and model number, evaluation report number and name or logo of the inspection
145 agency.

146 **2.1.5 Design Calculations:** Clear and comprehensive calculations of ASD or LRFD
147 structural capacities for the system or device, based on requirements of the IBC or UBC
148 and this criteria. Calculations shall be sealed by a registered design professional.

149 **2.2 Testing Laboratories:** Testing laboratories shall comply with Section 2.0 of the

150 ICC-ES Acceptance Criteria for Test Reports (AC85) and Section 4.2 of the ICC-ES
151 Rules of Procedure for Evaluation Reports.

152 **2.3 Test Reports:** Reports of tests required under Section 3.0 of this criteria shall
153 comply with AC85 and reporting requirements in referenced standards.

154 **2.4 Product Sampling:** Sampling of devices for tests under this criteria shall comply
155 with Section 3.1 of AC85.

156 **3.0 DESIGN, TEST, AND PERFORMANCE REQUIREMENTS:**

157 **3.1 General:** The belled segmented pipe foundation systems and devices shall be
158 evaluated for resistance to axial compression, axial tension, lateral loads or a combination
159 of these loads. Load types that are not applicable to the system shall be clearly
160 identified. The required capacities shall be evaluated by considering four primary
161 structural elements of the belled segmented pipe foundation system as shown in Figure
162 1. These element capacities are described as Bracket Capacity (P1), Segmented Pipe
163 Capacity (P2), Bullet Assembly Capacities (P3a and P3), and Soil Capacity (P4). The
164 allowable capacity of a belled segmented pipe foundation system or device shall be the
165 lowest value of P1, P2, P3, and P4, from each application illustrated in Figure 1. For
166 evaluation of belled segmented pipe foundation devices subject to combined lateral
167 loads and axial compression or axial tension, the allowable lateral capacity and
168 allowable axial capacities shall be determined and reported separately. The allowable
169 strength under combined load conditions shall be determined using the interaction
170 equation provided in the AISC referenced standard.

171 **3.2 P1 Bracket Capacity:** The P1 bracket capacity is the maximum load that can be

172 sustained by the bracket device of a belled segmented pipe foundation system
173 based on strength in accordance with Section 3.10.

174 **3.3 P2 Segmented Pipe Capacity:** The P2 segmented pipe capacity is the specified
175 load that can be sustained by the pipe and/or coupler elements of a belled segmented pipe
176 foundation system based on strength in accordance with Section 3.11.

177 **3.4 P3a and P3 Bullet Assembly Capacities:** The bullet assembly capacity is the
178 specified load that can be sustained by the bullet assembly of a belled segmented pipe
179 foundation system based on strength or deformation in accordance with Section 3.12.

180 **3.4.1 P3a Bullet Assembly Capacity:** The P3a bullet assembly capacity is the
181 specified load that can be sustained by the bullet assembly of the belled segmented
182 pipe foundation system during advancement of the segmented pipe sections prior to
183 expansion of the bullet assembly to its belled shape to provide additional bearing area.

184 **3.4.2 P3 Belled Bullet Assembly Capacity:** The P3 belled bullet assembly capacity
185 is the specified load that can be sustained by the belled bullet assembly of the belled
186 segmented pipe foundation system after the bullet assembly has expanded to its belled
187 shape and has been stabilized to its final increased bearing area.

188 **3.5 P4 Soil Capacity:** The P4 soil capacity is the specified load that can be
189 sustained by the soil or bedrock bearing stratum supporting the foundation system or
190 device based on strength and settlement or pullout in accordance with Section 3.13.

191 **3.6 Determination of Allowable Design Capacities:** In accordance with Section 3.7 and
192 Section 3.8, the allowable design capacities of belled segmented pipe foundation elements
193 P1, P2, P3 and P3a shall be evaluated based on conventional design with no testing

194 required, Special Analysis with verification tests, or solely on tests. All load tests shall
195 be conducted in accordance with Section 4.0. The allowable capacity P_4 shall be
196 determined by the registered design professional or through direct measurements as
197 specified in Section 3.13.

198 **3.7 Design Methods:**

199 **3.7.1 Conventional Design:** For conventional design of steel, either Allowable Stress
200 Design (ASD) or Load and Resistance Factor Design (LRFD) methods referenced in
201 the IBC or UBC, may be used to calculate the allowable design capacity, P' . For design of
202 concrete, strength design methods referenced in ACI 318 (IBC) or the UBC shall be used
203 to calculate the design capacity.

204 **3.7.1.1 ASD Method:** When using the ASD method, the allowable design capacity,
205 P' , shall be taken as the allowable strength, P_a , and shall be determined in
206 accordance with the applicable code or referenced standard (Eq-3).

$$207 \quad P' = P_a \text{ (ASD)} \quad \text{(Eq-3)}$$

208 **3.7.1.2 LRFD Method:** When using the LRFD method, the allowable design
209 capacity, P' , shall be taken as 0.7 times the design strength, ϕP_n , ϕP_n
210 determined in accordance with the applicable code or referenced standard (Eq-4).

$$211 \quad P' = 0.7\phi P_n \text{ (LRFD)} \quad \text{(Eq-4)}$$

212 **3.7.2 Special Analysis:** Where special analysis is used, the allowable capacity P' shall
213 be taken as 0.6 times the resistance based on yield strength (P_y) or, when stress
214 concentrations are prevalent, P' shall be 0.5 times the resistance based on maximum
215 strength (P_{max}) (Eq-5).

216
$$P' = 0.6P_y \text{ or } 0.5P_{max} \text{ (Special Analysis) (Eq-5)}$$

217 **3.7.3 Direct Measurement:** Where load testing only is used and the number of
218 samples is not specified, the allowable capacity shall be reported as the average
219 allowable strength determined in accordance with Section 4.0 from tests conducted on
220 at least five specimens, provided all test results are within 15 percent ($\pm 15\%$) of the
221 average. Otherwise, the allowable capacity from testing only shall be based on the
222 least test result. For direct measurement of belled segmented pipe foundation device
223 capacities, testing shall be conducted in accordance with the applicable test procedure
224 described in Section 4.0. The allowable capacity, P' shall be taken as 0.6 times the
225 resistance based on yield strength (P_y) or 0.5 times the maximum strength (P_{max}),
226 whichever yields the lowest value (Eq-6).

227
$$P' = 0.6P_y \text{ or } 0.5P_{max} \text{ (Direct Measurement) (Eq-6)}$$

228 For direct measurement of soil capacity, testing shall be conducted in accordance with
229 Section 4.4. For determination of allowable soil capacity, a factor of safety equal to 2
230 or greater shall be applied to the maximum measured soil capacity.

231 **3.8 Capacity Limits:** For conventional design, the maximum allowable design
232 capacity of belled segmented pipe foundation systems and devices is 40 kips (177.9 kN)
233 in axial tension, 50 kips (222.4 kN) in axial compression and 6 kips (26.7 kN) in
234 lateral resistance. Belled segmented pipe foundation systems or devices with
235 allowable design capacities greater than these normal capacity limits require special
236 analysis with additional verification testing as prescribed in Sections 3.10 to 3.13 or if
237 capacity is to be determined by testing alone, the number of test samples shall be

238 double the amount otherwise required.

239 **3.9 Corrosion:** Belled segmented pipe foundation systems and devices shall be bare
240 steel, powder-coated steel or zinc-coated steel. Powder coatings shall comply with
241 the ICC-ES Acceptance Criteria for Corrosion Protection of Steel Foundation
242 Systems Using Polymer (EAA) Coatings (AC228), and the coating thickness shall be at
243 least 450 μm (0.018 inch). Zinc coatings shall comply with ASTM A 123, A 153, B 633,
244 or B 695, as applicable. Loss in steel thickness due to corrosion shall be accounted
245 for in determining structural capacities by reducing the thickness of all belled
246 segmented pipe foundation components by the sacrificial thickness over a period, t ,
247 of 50 years. The design thickness, T_d , of belled segmented pipe foundation
248 components used in capacity calculations and testing shall be computed by Eq-6. For
249 purposes of design calculations and fabrication of test specimens, the thickness of each
250 component shall be reduced by $1/2 T_s$ on each side, for a net reduction in thickness of
251 T_s .

$$252 \quad T_d = T_n - T_s \quad (\text{Eq-6})$$

253 where T_n is nominal thickness and T_s is sacrificial thickness ($t = 50$ yrs).

$$254 \quad T_d \sim \text{base-steel thickness}$$

$$255 \quad \text{Zinc-coated steel: } T_s = 25 t^{0.65} = 318 \mu\text{m} (0.013 \text{ in})$$

$$256 \quad \text{Bare steel, } T_s = 40 t^{0.80} = 915 \mu\text{m} (0.036 \text{ in})$$

$$257 \quad \text{Powder coated steel, } T_s = 40(t-16)^{0.80} = 671 \mu\text{m} (0.026 \text{ in})$$

258 For bare steel and powder-coated steel, T_n shall be the base-steel thickness. For zinc-
259 coated steel, T_n may be the sum of the base-steel thickness and zinc coating thickness,

260 provided the minimum zinc coating thickness is 86 μm (0.0034 in). Otherwise, the
261 sacrificial thickness, T_s , shall be determined by linear interpolation between bare steel
262 and zinc coated steel using the actual specified zinc coating thickness.

263 For powder-coated steel, the life of powder coating is taken as 16 years, maximum.
264 Hence, t has been reduced by 16 in the determination of T_s .

265 For verification of Special Analysis or for determination of allowable capacity through
266 testing only, test specimens shall be constructed using steel thickness equal to T_d .
267 Alternatively, unaltered test specimens may be used and the resulting allowable strength
268 shall be reduced by multiplying the result by a scaling factor that takes into account
269 corrosion and the observed failure mode. Thus, a tension failure result shall be scaled by
270 the area of the fracture surface, while a flexural failure would be scaled by the reduced
271 section modulus. The testing laboratory shall determine the appropriate scaling method
272 and identify the failure mode.

273 Corrosion loss shall be accounted for regardless of whether devices are below or
274 above ground or embedded in concrete. Zinc-coated steel and bare steel components shall
275 not be combined in the same system. Powder coated steel may be combined with zinc-
276 coated steel and bare steel components. All belled segmented pipe foundation
277 components shall be galvanically isolated from concrete reinforcing steel, building structural
278 steel, or any other metal building components.

279 **3.10 P1 Bracket Capacity:** Belled segmented pipe foundation brackets shall be
280 classified as side vertical load as illustrated in Figure 1. At a minimum, evaluation of P1
281 shall include determination of strength of the connection of the bracket to the structure,

282 the internal strength of the bracket itself, and the strength of connection of the bracket to
283 the uppermost segmented pipe section. The frictional resistance of concrete on a
284 horizontal bracket component shall be determined using a coefficient of friction of 0.4 or
285 less. The shear strength of concrete also shall be calculated in accordance with the
286 applicable code. Brackets may be evaluated for compression, tension, and/or lateral
287 strengths. If not vertical, the angle of the segmented pipe section with respect to the
288 bracket shall be accounted for in the calculations. The uppermost segmented pipe
289 section and the bracket shall be attached to each other and to the structure by
290 mechanical connections. Installation shall be limited to support of uncracked concrete, as
291 determined in accordance with the applicable code. In order for the segmented pipe
292 sections to be considered sidesway braced, the structure shall provide lateral restraint
293 to the segmented pipe sections equal to or greater than 0.4 percent of the segmented
294 pipe section's allowable axial compression load.

295 **3.10.1 Side Load:** Side load brackets are illustrated in Figure 1 and support tensile or
296 compressive loads that are not concentric with the primary axis of the segmented
297 pipe sections.

298 **3.10.1.1 Eccentricities:** Rotational moments caused by vertical load eccentricity shall
299 be subdivided into two components, bracket eccentricity and structure eccentricity, as
300 illustrated in Figure 2. The segmented pipe sections and the connected bracket
301 components, consisting of the connected bracket, connection of the bracket to the
302 segmented pipe, and connection of the bracket to the structure, shall resist bracket
303 eccentricity. Structure eccentricity varies with application and is generally resisted by
304 the internal strength of the structure to which the bracket is attached. Therefore, resistance

305 to structure eccentricity shall be determined on a case-by-case basis. For purposes of
306 bracket eccentricity and internal strength design, the location of the resultant vertical
307 compression force of the concrete structure on an angle bracket shall be taken as the
308 centroid of an area defined by the uniform concrete bearing stress as shown in Figure 2.

309 **3.10.1.2 Structure Bracing:** Side load brackets shall only be used to support
310 structures that are braced as defined in IBC Section 1808.2.5.

311 **3.10.2 Analysis:** The strength of connected bracket components and segmented pipe
312 sections shall be evaluated based on a static analysis transferring the pipe load through
313 the bracket into the existing structure. The analysis shall also include forces induced
314 from the application of pressure on the segmented pipes for advancement of the
315 segmented pipe sections during installation.

316 **3.10.2.1 Connection to the Structure:** Axial compression, axial tension, or
317 lateral load connection capacities shall be determined in accordance with the IBC, UBC,
318 or a current ICC-ES evaluation report. For purposes of evaluation, the structure shall be
319 modeled as a mass of structural plain concrete, semi-infinite in extent, with varying strength.
320 The structure shall be assumed to be fixed in translation and rotation, but can move
321 freely in the vertical direction. At a minimum, design of the connection shall be based on
322 normal-weight concrete with a specified compressive strength of 2,500 psi (17.22
323 MPa). Other concrete strengths, structural lightweight concrete, masonry and other
324 materials also can be included in the evaluation at the option of the bracket manufacturer.
325 For all combinations of concrete strength and/or material compositions, details regarding
326 connection of the bracket to the structure types (i.e., anchor bolt placement, grouting,

327 surface preparation, etc.) shall be prescriptively specified.

328 **3.10.3 Test Requirements:** Except as stipulated in Section 3.10.3.1, verification tests shall
329 not be required for evaluation of foundation brackets provided all analysis is accomplished
330 using Conventional Design as set forth in Section 3.7 and allowable capacities are within
331 the range of Normal Capacity Limits as set forth in Section 3.8. A minimum of three
332 verification load tests shall be conducted in each load direction (axial compression,
333 axial tension, and lateral) on any component of a bracket or bracket / segmented pipe
334 system evaluated using Special Analysis and for brackets exceeding Normal Capacity
335 Limits. Where tests are required for verification of lateral resistance, tests shall be
336 conducted to verify lateral resistance in all directions for which lateral resistance is being
337 claimed. Where testing alone is used without analysis, a minimum of five tests shall be
338 conducted in accordance with Section 3.7.3. Bracket tests shall be conducted in
339 accordance with Section 4.1 for compression and tension and Section 4.4.2 for
340 lateral resistance.

341 **3.10.3.1 Friction Connection of Segmented Pipe to Bracket:** When the segmented
342 pipe connection to the bracket is dependent on friction developed by a clamping force,
343 testing shall be required. Testing shall be in accordance with Section 4.1.

344 **3.11 P2 Segmented Pipe Capacity:** At a minimum, segmented pipe capacities shall
345 be evaluated for axial compression. Segmented pipe sections may also be evaluated for
346 axial tension and lateral resistance, with consideration of combined lateral and axial
347 loading. Evaluation of segmented pipe sections shall include connections between
348 segmented pipes and cementitious fill. All segmented pipe section connections shall be

349 made via a mechanical coupler. When cementitious fill is used, the system shall be
350 evaluated for both filled and unfilled conditions as appropriate.

351 **3.11.1 Tension:** Evaluation of segmented pipe sections for tension shall include yielding
352 on the gross area and fracture at any coupler. At couplers, there shall be consideration of
353 all stresses and failure modes, including increased stresses at bolt holes and welds within
354 the coupler as well as for the attachment of the coupler to the pipe section. Concrete fill
355 shall not be considered to contribute to the tension capacity. Reinforcing steel within the
356 pipe and concrete fill may be considered if shown to be adequately connected with load
357 transfers in accordance with the applicable codes.

358 **3.11.2 Compression:** Evaluation of segmented pipe sections for compression shall
359 include buckling resistance, yielding on the gross area and yielding at any couplers. At
360 couplers, there shall be consideration of all stresses and failure modes, including
361 increased stresses at bolt holes and welds within the coupler as well as for the attachment
362 of the coupler to the pipe section. A bending moment shall be applied to the top of the
363 segmented pipe section in buckling calculations in accordance with Section 3.11.2.3.

364 **3.11.2.1 Unsupported Length:** Unsupported segmented pipe lengths shall include
365 the length of the segmented pipe sections in air, water, or in fluid soils. For unbraced
366 systems, the lengths specified in IBC Section 1808.2.9.2 shall apply unless determined
367 otherwise by special analysis. In accordance with IBC Section 1808.2.9.1, any soil
368 other than fluid soil shall be deemed to afford sufficient lateral support to prevent
369 buckling of systems that are braced. Bracing shall comply with IBC Section 1808.2.5.
370 Firm soils shall be defined as any soil with a Standard Penetration Test blow count of

371 five or greater. Soft soils shall be defined as any soil with a Standard Penetration Test
372 blow count greater than zero and less than five. Fluid soils shall be defined as any soil with
373 a Standard Penetration Test blow count of zero [weight of hammer (WOH) or weight of
374 rods (WOR)]. Standard Penetration Test blow count shall be determined in
375 accordance with ASTM D 1586.

376 **3.11.2.2 Effective Length:** Effective lengths shall be determined using the unsupported
377 length defined in Section 3.11.2.1 and the appropriate effective length factor, K ,
378 determined in accordance with the AISC referenced standard. Slenderness ratio
379 limitations as specified by the AISC referenced standards do not apply.

380 **3.11.2.3 Coupler Rigidity:** Coupler rigidity shall be considered for all cases except
381 braced systems in firm or soft soils. To account for coupler rigidity, the eccentricity of the
382 axial compressive load applied to the segmented pipe sections shall be increased by a
383 distance, nAe_c , where n is the number of couplers possible in the unsupported length and
384 e_c is the maximum lateral deflection of the unsupported length of segmented pipe
385 sections due to flexure of the coupler under an applied lateral load of 0.4 percent of the
386 applied axial compressive load. [Maximum lateral deflection of the segmented pipe
387 sections due to coupler flexure shall be determined in accordance with Section 4.2.3].

388 **3.11.3 Lateral Resistance:** Lateral resistance of the segmented pipe sections is
389 necessarily coupled with soil capacity and shall be determined in accordance with Section
390 3.13. Segmented pipe section area, moment of inertia, and elasticity shall be used as
391 inputs in the analysis. Maximum bending moment and shear stress determined from the
392 analysis shall be limited by the allowable bending and shear resistance of the

393 segmented pipe section or the segmented pipe section couplers, whichever is less.

394 Deflection of segmented pipe section couplers shall be included in lateral resistance
395 analysis.

396 **3.11.4 Elastic Shortening or Lengthening:** Methods (equations) shall be provided
397 for estimation of elastic shortening/lengthening of the segmented pipe sections under the
398 allowable axial load plus any slip in the couplers. These methods shall be based upon
399 conventional design.

400 **3.11.5 Combined Stresses:** Segmented pipe section evaluation shall include
401 combined stresses. Combinations of tension, compression, bending, and lateral loads
402 shall be considered as applicable.

403 **3.11.6 Test Requirements:** Verification tests shall not be required for evaluation of
404 segmented pipe section tension, compression, and bending moment provided all analysis is
405 accomplished using conventional design in accordance with Section 3.1 and allowable
406 capacities are within the range of normal capacity limits as set forth in Section 3.8. A
407 minimum of three verification load tests shall be conducted on separate specimens in
408 each direction (compression, tension, bending) on any component of segmented
409 pipe sections evaluated using special analysis, and for segmented pipe sections that
410 exceed normal capacity limits as set forth in Section 3.8. Where testing alone is used
411 without analysis, a minimum of five tests shall be conducted in accordance with Section
412 3.7.3. Tests are required to determine coupler rigidity as described in Sections 3.11.2.
413 Tests for segmented pipe sections capacity shall be conducted in accordance with
414 Section 4.2.

415 **3.12 P3 Bullet Assembly Capacity:** Bullet assembly capacity and weld strength
416 shall be evaluated for compression and tension (if applicable). The bullet assembly
417 shall be evaluated for both during advancement through the soil P3a and the belled
418 condition P3.

419 **3.12.1 P3a Bullet Assembly Expansion:** Expansion of the bullet assembly shall be
420 determined by testing in accordance with Section 4.3.1. A minimum of five samples, with
421 an equal number of samples from three or more separate production batches, shall be
422 used in the testing. The mean ultimate (maximum) applied axial force to cause
423 expansion to the belled shape shall be determined from the test population.

424 **3.12.2 P3 Belled Bullet Assembly:** The bullet assembly belled capacity shall be
425 determined after stabilization, by testing in accordance with Section 4.3.2. A minimum of
426 five samples, with an equal number of samples from three or more separate production
427 batches, shall be used in the testing. These may be the same samples used for testing
428 specified in Section 3.12.1. The mean ultimate (maximum) axial resistance force shall
429 be determined from the test population.

430 **3.12.2.1 Stabilization by Cementitious Material:** When a cementitious material is
431 used to stabilize the belled shape of the bullet assembly, compression testing of each
432 material used for stabilization shall be conducted and reported.

433 **3.12.3 Lateral Capacity:** Lateral capacity of the bullet assembly shall not be
434 considered.

435 **3.12.4 Test Requirements:** Each bullet assembly with differing properties, for which
436 evaluation is being sought, shall be tested. The allowable capacity for each size and

437 type of bullet assembly shall be reported as the average result of at least five test
438 specimens. In order to allow the mean values, individual results determined from testing
439 shall be within 15 percent of the average of tests. Otherwise, the least test result shall
440 apply. Bullet assembly tests shall be conducted in accordance with Section 4.3.

441 **3.13 P4 Soil Capacity:** Soil capacity includes the tension, compression, and/or lateral
442 resistance of a belled segmented pipe foundation embedded in ground, as applicable.

443 **3.13.1 Axial Capacity Verification:** For all belled segmented pipe foundation
444 systems, full-scale field installation and load tests shall be conducted to verify the axial
445 capacity on specimens installed to the resistance required to expand the bullet
446 assembly to its increased bearing area determined in accordance with Section 3.11.3.
447 The tests shall be regarded as a successful verification of installation and allowable
448 capacity, provided all full-scale axial load tests exceed the allowable capacity of the
449 system by a factor of safety of at least 2.0.

450 At least two specimens of each type of segmented pipe sections shall be tested in
451 each load direction (tension or compression) for which evaluation is being sought.
452 Variations in foundation device size and material strengths shall constitute a different
453 type of specimen. Two separate specimens shall be tested in each direction
454 (compression and/or tension) for which evaluation is being sought. Test specimens shall
455 consist of a minimum of a bullet assembly, two pipe sections and at least one coupler.
456 The test specimen may include a bracket. All verification tests shall be conducted at
457 sites described in Section 3.13.4. Additional information on testing is provided in Section
458 4.4.1. The determination of soil capacity, P4, on any specific site or with any
459 configuration of belled segmented pipe foundation devices other than the test site and

460 test specimen is outside the scope of this acceptance criteria. The evaluation report
461 shall indicate that soil capacity shall be determined by a registered design professional
462 for each site considering groundwater and other geotechnical conditions. As an
463 alternative, load capacity correlations for specific soil conditions may be determined in
464 accordance with Section 3.13.2.

465 **3.13.2 Load Capacity Correlations:** Evaluation reports may include a correlation
466 between measured axial force at which bullet assembly expansion occurs, the ratio of
467 the square of the diameter of the expanded bullet assembly to the square of the
468 diameter of the bullet assembly during advancement and the ultimate soil capacity.
469 This correlation shall be based on the results of testing the bullet assembly as outlined
470 in Section 3.12.4 and full-scale field tests as outlined in Section 3.13.1.

471 **3.13.3 Lateral Resistance:** Allowable soil capacity in the lateral direction shall be
472 determined through load tests on specimens installed in different soil conditions. The
473 allowable soil capacity shall be determined based on deflection criteria set forth in
474 Section 4.4.2. In order to be valid, allowable capacities determined for each type of
475 specimen in each soil type shall be within 15 percent of the average allowable capacity
476 for those tests.

477 A minimum of four specimens of each type of segmented pipe section shall be tested
478 in each soil type for which evaluation is being sought. Variations in segmented pipe
479 section size, geometry, total length and material strength shall constitute a different type
480 of specimen. Variations in bullet assembly size, geometry or material strength does not
481 require separate tests. Four separate specimens shall be tested in each transverse
482 direction for which evaluation is being sought if the segmented pipe section is not axially

483 symmetric. Test specimens shall consist of a segmented pipe section, at least one
484 coupler located with the manufacturer's smallest extension length from the ground
485 surface and a bullet assembly. The test may include a bracket. The subsurface profile at
486 all test sites shall be characterized in a soil investigation by a registered design
487 professional. Additional information on testing is provided in Section 3.13.4. Allowable
488 soil capacity for different specimens in different soil categories shall be tabulated in the
489 evaluation report. The evaluation report shall contain a statement that soil capacity for
490 lateral resistance in soil conditions that substantially differ from actual test sites included
491 in the evaluation shall be determined by a registered professional engineer on a
492 case-by-case basis.

493 **3.13.4 Test Requirements:** Axial compressive, tensile, and lateral allowable load
494 capacity shall be verified through field load tests as provided in Section 3.13. At least
495 two verification tests are required for axial compression and at least two verification
496 tests are required for axial tension. If a load capacity correlation is specified, then at
497 least four tests are required for axial compression verification and at least four tests are
498 required for axial tension verification for each segmented pipe section size for which
499 evaluation is being sought. The two verification tests required for compression and
500 tension may be included in the tests for load capacity correlations. If evaluation of lateral
501 resistance is requested, four verification tests are required for each segmented pipe
502 section size, segmented pipe section geometry, and soil type.

503 Tests for axial compression and tension soil capacity shall be conducted in
504 accordance with Section 4.4.1 and tests for lateral resistance shall be conducted in
505 accordance with Section 4.4.2. Tension and compression verification load tests are

506 required to be conducted at the facility or field station of a testing laboratory complying
507 with Section 2.2. The subsurface profile at other test sites shall be characterized in a
508 soil report by a registered design professional. Subsurface profile characterization shall
509 include soil borings, standard penetration resistance tests, and basic laboratory
510 classification tests essential for soil classification according to the Unified Soil
511 Classification System. All field penetration tests, laboratory tests, and soil classifications
512 shall be conducted in accordance with ASTM D 1586.

513 **4.0 TEST METHODS**

514 **4.1 P1 Bracket Capacity:** Where specified herein, each size and configuration of the
515 bracket shall be tested. The configuration of the bracket and direction of applied loads in
516 the test apparatus shall be as close to actual field conditions as practical. Pertinent data
517 such as maximum load applied, maximum bracket rotation, failure mode, etc., shall be
518 reported.

519 **4.1.1 Side Load:**

520 **4.1.1.1 Setup:** Compression and tension tests can be conducted in a horizontal
521 configuration, as illustrated in Figure 3. The bracket shall be mounted to a block of plain
522 concrete of known strength that is fixed with respect to translation and rotation. The
523 connection of the bracket to the concrete shall be in accordance with manufacturer's
524 installation instructions. Load shall be applied to the bracket using a standard length
525 section of segmented pipe section secured to the bracket in a manner that duplicates
526 actual field conditions. The loaded end of the segmented pipe section shall be
527 rotationally fixed. Axial load shall be applied in the direction of the longitudinal axis of

528 the segmented pipe section. Any eccentricity inherent in the bracket configuration and
529 angle of the segmented pipe section to bracket shall be accounted for and shall be
530 modeled to match the anticipated design purpose.

531 **4.1.1.2 Procedure:** Axial deflection shall be recorded as a function of applied load
532 at regular intervals equal to or less than 20 percent of the anticipated allowable load.
533 The rate of load application shall be sufficiently slow to simulate static conditions. Each
534 load increment shall be held for a minimum of one minute. Yield strength and ultimate
535 (maximum) strength of the bracket shall be determined using conventional analysis of a
536 plot of load versus deflection. The allowable strength of the bracket shall be determined
537 from yield or ultimate (maximum) strength using the equations provided in Section 3.7.3,
538 whichever formula results in the lowest value. Compression tests shall be conducted
539 within 24 hours of the bracket test on concrete cylinders cast at the same time as the
540 test specimen to establish concrete compressive strength. Cylinders shall be stored and
541 cured according to Section 9.3.1 of ASTM C 31 (field cure). The tested concrete
542 compressive strength shall be within 15 percent of the specified compressive strength.
543 Concrete cylinder compression tests shall be conducted in accordance with ASTM C
544 39.

545 **4.2 P2 Segmented Pipe Section Capacity:**

546 **4.2.1 Axial Tension and Compression:**

547 **4.2.1.1 Setup:** Compression and/or tension tests shall be conducted on two
548 sections of segmented pipes with a coupler located between the pipe section
549 specimens. When filled pipe sections are specified, tests shall be performed for both

550 hollow and filled sections. Additionally, standard compression tests shall be performed
551 on specimens of the fill material in accordance with ASTM standards. The test
552 specimens shall be mounted to a vertical or horizontal load frame with one end attached
553 to a fixed platform and the other end attached to a mobile platform with the capability to
554 apply the load to the specimen in the axial direction. The coupler connection shall be
555 done in accordance with manufacturer's specific published recommendations. Direction
556 of loading shall be coaxial with the longitudinal axis of the segmented pipe sections. The
557 testing apparatus shall provide sufficient rigidity as to minimize any slip or deformation
558 not associated with the test specimens.

559 **4.2.1.1.1 Setup for Laterally Braced Belled Segmented Pipe Foundation**

560 **System:** To evaluate braced buckling resistance, compression specimens shall be
561 evaluated to simulate segmented pipe section advancement through the upper layers of
562 soil, with a total specimen length of 5 feet. All other testing shall be in accordance with
563 Section 4.2.1.1.

564 **4.2.1.1.2 Setup for Laterally Unbraced Belled Segmented Pipe Foundation**

565 **System:** To evaluate unbraced buckling resistance, compression specimens shall have
566 enough segmented pipe sections to achieve the minimum unbraced length equal to or
567 greater than the effective length as specified in Section 3.11.2.2. All other testing shall
568 be in accordance with Section 4.2.1.1.

569 **4.2.1.2 Procedure:** Loads shall be applied to the specimen in increments not
570 exceeding 20 percent of the design allowable load of the specimen. Each load
571 increment shall be held for a minimum of one minute. The specimen shall be loaded to
572 failure. Application of the load shall be performed at a slow rate to simulate a statically

573 applied load. Pertinent data such as maximum load applied, maximum segmented pipe
574 section or connection deformation, failure mode, etc., shall be reported. Yield strength
575 and ultimate (maximum) strength of the segmented pipe sections and coupler shall be
576 determined using conventional analysis of a plot of load versus deflection. The
577 allowable strength of the segmented pipe sections and coupler shall be determined from
578 yield or ultimate (maximum) strength and the equations provided in Section 3.7.3,
579 whichever equation results in a lower value.

580 **4.2.2 Bending:**

581 **4.2.2.1 Setup:** Bending tests shall be conducted on a segmented pipe section that
582 is horizontally arranged in a compression load frame, as illustrated in Figure 4. For
583 segmented pipe sections with a non-circular cross section, as a minimum, the tests
584 shall be conducted with the least resistant orientation. The distance between
585 segmented pipe section supports shall be at least 36 inches (914 mm) or 12 times the
586 maximum outside cross-sectional dimension of the segmented pipe section, whichever
587 is greater. A coupler shall be located approximately in the center of the specimen.
588 Loads shall be applied using a two point test where the load points straddle the coupler
589 so that a uniform bending moment is produced in the coupler.

590 **4.2.2.2 Procedures:** Load shall be applied and deflections measured at intervals of
591 less than or equal to 20 percent of the load corresponding to the theoretical allowable
592 bending moment. Application of load shall be performed at a slow rate to simulate a
593 statically applied load. Pertinent data such as maximum load applied, maximum
594 segmented pipe section or coupler deformation, failure mode, etc., shall be reported.

595 Yield strength and ultimate (maximum) strength of the segmented pipe section and
596 coupler shall be determined using conventional analysis of a plot of load versus
597 deflection. The allowable bending strength of the segmented pipe section and coupler
598 shall be determined from yield or (maximum) strength and the equations provided in
599 Section 3.7.3, whichever equation results in a lower value.

600 **4.2.3 Coupler Rigidity:**

601 **4.2.3.1 Setup:** The maximum lateral deflection of segmented pipe sections due to
602 coupler flexure shall be determined using segmented pipe sections with a length equal
603 to the unsupported length [60 or 120 inches (1524 or 3048 mm) as specified by Section
604 1808.2.9.2 of the IBC]. The segmented pipe sections shall have the maximum number
605 of couplers possible over its length based on the standard length of segmented pipe
606 sections. The segmented pipe sections shall be horizontally or vertically arranged in a
607 load frame at the evaluation report applicant's option with one end fixed and the other
608 end unsupported, as illustrated in Figure 5. A load shall be applied perpendicularly to
609 the unsupported end of the segmented pipe section.

610 **4.2.3.2 Procedures:** A vertical load equal to 0.4 percent of the allowable
611 compression load on the belled segmented pipe foundation segmented pipe section
612 system shall be applied. The total deflection of the loaded end of the segmented pipe
613 section, including any free deflection, shall be measured relative to a horizontal plane
614 extending from the fixed end. The total deflection shall be reported and used in
615 segmented pipe section eccentricity computations.

616 **4.2.4 Shear Strength:**

617 **4.2.4.1 Setup:** The maximum shear strength of segmented pipe sections and
618 couplers shall be determined using specimens with lengths as appropriate for the test
619 apparatus. The specimen shall be horizontally or vertically arranged in a load frame with
620 one end fixed and the other end free. A load shall be applied normal to the segmented
621 pipe or coupler using a roller or slide to avoid inducing flexure into the system.

622 **4.2.4.2 Procedure:** The loads shall be applied in increments not exceeding 20
623 percent of the allowable shear load on the segmented pipe section or coupler. The total
624 deflection of the segmented pipe section or coupler at the point of load application shall
625 be measured at each increment. Load shall be applied at a slow rate to simulate
626 statically applied load. Each load increment shall be held for a minimum of one minute.
627 Yield and ultimate (maximum) strength of the segmented pipe or coupler shall be
628 determined using a conventional analysis of a plot of load versus deflection.

629 **4.3 P3 and P3a Bullet Assembly Capacity:** Where specified herein, each size and
630 configuration of bullet assembly shall be tested.

631 **4.3.1 P3a Bullet Assembly:**

632 **4.3.1.1 Setup:** Compression tests shall be conducted on each bullet assembly prior
633 to expansion to its belled shape. The test specimen shall be placed in a load frame with
634 the top of the bullet assembly snug against a fixed platform and the base placed on a
635 mobile platform with the capability to apply the load to the specimen in the axial
636 direction. The testing apparatus shall provide sufficient rigidity so as to minimize any slip
637 or deformation not associated with the test specimens.

638 **4.3.1.2 Procedures:** Loads shall be applied to the specimen in either continuously
639 increasing increments, or in increments not exceeding 10 percent of the design ultimate
640 capacity. If the load is applied in continuously increasing increments, the rate of loading
641 (load increase per second) should be equal to the design ultimate capacity divided by
642 120 to 150. If the load is applied in increments, each load increment should be held for
643 30 to 60 seconds, and application of the load shall be at a moderate rate to simulate
644 actual field loading conditions. Pertinent data such as maximum load applied, maximum
645 bullet deformation, failure load, etc., shall be reported.

646 **4.3.2 P3 Belled Bullet Capacity for Compression:**

647 **4.3.2.1 Setup:** Compression tests shall be conducted on each stabilized belled
648 bullet assembly. The previously tested specimens (Section 4.3.1) may be stabilized
649 and tested. Where the bullet assembly is stabilized with hydraulic cement, grout, or
650 concrete, the materials shall reach 100 percent of their design compressive strength
651 prior to testing. The stabilized test specimen shall be placed in a load frame with the
652 top of the bullet assembly snug against a fixed platform and the base placed on a
653 mobile platform with the capability to apply the load to the specimen in the axial
654 direction. The testing apparatus shall provide sufficient rigidity as to minimize any slip or
655 deformation not associated with the test specimens.

656 **4.3.2.2 Procedure:** Loads shall be applied to the specimen in either continuously
657 increasing increments, or in increments not exceeding 10 percent of the design ultimate
658 capacity. If the load is applied in continuously increasing increments, the rate of loading
659 (load increase per second) should be equal to the design ultimate capacity divided by

660 120 to 150. If the load is applied in increments, each load increment should be held for
661 30 to 60 seconds, and application of the load shall be performed at a moderate rate to
662 simulate actual field loading conditions. Pertinent data such as maximum load applied,
663 maximum bullet deformation, failure mode, etc., shall be reported.

664 **4.3.3 P3 Belled Bullet Capacity for Tension:**

665 **4.3.3.1: Setup:** Tension tests shall be conducted on each stabilized belled bullet
666 assembly for which recognition for tension is desired. Where the bullet assembly is
667 stabilized with hydraulic cement, grout, or concrete, the materials shall reach 100
668 percent of their design compressive strength prior to testing. The stabilized test
669 specimen shall be mounted in a vertical or horizontal load frame with the expanded end
670 attached to a fixed platform, and the other end attached to a mobile platform with the
671 capability to apply the load to the specimen in the axial direction. The expanded end
672 should be encased in a high-strength concrete mold with a steel backer plate, designed
673 to impart the expected reaction force of soil against the expanded bullet assembly
674 during uplift. A bond break should be placed on the outside of the expanded bullet
675 assembly to prevent adhesion of the concrete to the bullet. Direction of loading shall be
676 coaxial with the longitudinal axis of the bullet assembly. The testing apparatus shall
677 provide sufficient rigidity as to minimize any slip or deformation not associated with the
678 test specimens.

679 **4.3.3.2 Procedure:** Loads shall be applied to the specimen in either continuously
680 increasing increments, or in increments not exceeding 10 percent of the design ultimate
681 capacity. If the load is applied in continuously increasing increments, the rate of loading

682 (load increase per second) should be equal to the design ultimate capacity divided by
683 120 to 150. If the load is applied in increments, each load increment should be held for
684 30 to 60 seconds, and application of the load shall be performed at a moderate rate to
685 simulate actual field loading conditions. Pertinent data such as maximum load applied,
686 maximum bullet deformation, failure mode, etc. shall be reported.

687 **4.4 P4 Soil Capacity:**

688 **4.4.1 Full-scale Load Tests:**

689 **4.4.1.1 Setup:** Full-scale load tests shall be conducted in accordance with ASTM D
690 1143 for axial compression and ASTM D 3689 for axial tension. The quick load test
691 procedure set forth in Section 5.6 of ASTM D 1143 shall be used in compression tests.
692 Installation of the belled segmented pipe sections shall be done in accordance with the
693 installation instructions. The brand, model number, and maximum capacity of the
694 installation device shall be reported. All test segmented pipe sections shall be installed
695 as close to vertical as possible. Pertinent data such as belled segmented pipe foundation
696 depth and final installation load achieved shall be reported. Load should be measured
697 with a calibrated jack. Calibration of jacks shall be performed on equipment whose
698 calibration is traceable back to NIST (National Institute of Standards and Technology).
699 For tension tests, the belled segmented pipe foundation shall be installed such that the
700 minimum depth from the ground surface to the bottom of the bullet assembly is $24D$, where
701 D is the diameter of the expanded belled bullet assembly.

702 **4.4.1.2 Procedures:** Direction of loading shall be coaxial with the longitudinal axis
703 of the segmented pipe sections. Application of load shall be done at a slow rate to

704 simulate a statically applied load. Segmented pipe sections shall be installed until the
705 bullet assembly expands to its belled shape. Maximum load capacity shall be that which
706 is achieved when plunging of the belled bullet assembly occurs or when total deflection
707 exceeds 1 inch (25.4 mm), whichever occurs first.

708 **4.4.2 Lateral Load Tests:**

709 **4.4.2.1 Setup:** Lateral load tests shall be conducted in accordance with ASTM D
710 3966. These tests can be performed in two ways. If verification of lateral resistance of
711 brackets is required, the test setup shall consist of a belled segmented pipe foundation
712 representative of a standard installation with a bracket above the ground surface. The
713 bracket shall be connected to a structure constructed from wood, steel, or concrete
714 depending on the particular detail for which evaluation is being sought. The test setup
715 shall be such that lateral load is applied to the structure being supported immediately
716 above the bracket elevation. The tests shall be conducted with a free head arrangement in
717 accordance with ASTM D 3966. Where the bracket is intended to support a structure that
718 is rotationally restrained, the test may be conducted using fixed head or free head
719 arrangements in accordance with ASTM D 3966.

720 If verification of bracket capacity is not required, as in the case of conventional
721 design, then the tests shall be conducted with the segmented pipe section extending a
722 minimum of 12 inches (304.8 mm) from the ground surface. The lateral load shall be
723 applied to the segmented pipe section immediately above the ground surface. Depending
724 on whether the segmented pipe section is intended to support a structure that is
725 rotationally restrained, the test may be conducted using fixed head or free head

726 arrangements in accordance with ASTM D 3966.

727 Bracket and segmented pipe section installation shall be done in accordance with
728 the standards set forth in manufacturer's specific published recommendations. All test
729 segmented pipe sections shall be installed within the manufacturer's specified tolerances
730 for angle of installation for the bracket type. The minimum depth of the bullet assembly
731 shall be 180 inches (4572 mm).

732 **4.4.2.2 Procedures:** For tests including brackets or pipe sections that are
733 nonsymmetrical, separate specimens shall be loaded in all lateral directions for which
734 evaluation is being sought. Application of load shall be done at a slow rate to simulate a
735 statically applied load. The allowable load capacity reported shall be equal to half the load
736 required to cause $\frac{3}{4}$ inch (19.1 mm) of lateral deflection at the ground surface.

737 **4.5 General Testing Requirements:** Test equipment shall be adequate to impose
738 anticipated maximum loads. If loading is not carried to failure, the highest value achieved
739 will be considered the maximum load.

740 **5.0 QUALITY CONTROL**

741 **5.1 Manufacturing:** All products shall be manufactured under an approved quality
742 control program with inspections by an inspection agency accredited by the International
743 Accreditation Service (IAS) or otherwise acceptable to ICC-ES.

744 **5.2 Quality Control Documentation:** Quality documentation complying with the
745 ICC-ES Acceptance Criteria for Quality Documentation (AC10) shall be submitted.

746 **6.0 EVALUATION REPORT RECOGNITION**

747 **6.1 General:** The evaluation report shall include a description of the belled

748 segmented pipe foundation device or system, typical applications and limitations. The
749 evaluation report shall state that (1) the device or system shall be limited to support of
750 structures in IBC Seismic Design Categories A, B, and C or UBC Seismic Zones 0, 1,
751 and 2, only; (2) and the device or system shall not be used in conditions that are
752 indicative of a potential pile corrosion situation such as might be brought about by soil
753 resistivity less than 1,000 ohm-cm, pH less than 5.5, soils with high organic content,
754 sulfate concentrations greater than 1,000 ppm, landfills, or mine waste.

755 System and device descriptions shall include the dimensions of primary
756 components as well as engineering drawings of the product. Any bracket connections
757 to structures shall be prescriptively specified in construction details, including type and
758 condition of structure to be supported, drill holes, bolts, washer plates, field welds,
759 minimum concrete cover, concrete reinforcement, and leveling grout, as applicable.
760 The recommended angle of segmented pipe section installation and maximum
761 permissible departure from that angle shall be specified for each bracket. Construction
762 details for bracket connections shall indicate that materials with different corrosion
763 protection coatings shall not be combined in the same system and that belled
764 segmented pipe foundation devices and systems shall not be placed in electrical contact
765 (galvanically isolated) with structural steel, reinforcing steel, or any other metal building
766 components.

767 A table of allowable capacities (tension, compression, and/or lateral) for all elements
768 (P1, P2, P3, and P4, as applicable) shall be provided, with listings for each system or
769 device and all possible combinations and configurations. The evaluation report shall
770 state that the allowable capacity of a belled segmented pipe foundation device or

771 system shall be governed by the least allowable capacity, P1 through P4, as
772 applicable.

773 If lateral resistance is included in the evaluation report, a table of soil capacity in the
774 lateral direction based on load tests shall be provided for each type of segmented pipe
775 section in each test soil condition. The evaluation report shall indicate that soil capacity
776 in the lateral direction needs to be determined by a registered design professional
777 unless the soil conditions for the site in question are generally consistent with soil types
778 described in the evaluation report. For any belled segmented pipe foundation device
779 subject to combined lateral and axial compression or axial tension, the evaluation report
780 shall contain the maximum allowable lateral strength and the maximum allowable axial
781 strength and shall state that the strength of the device is governed by the interaction
782 equation given in the AISC reference standard.

783 The evaluation report shall provide a discussion of elastic shortening/lengthening,
784 anticipated settlements, and typical elastic deflections, as applicable, depending on the
785 end use. The discussion shall contain design values from analysis or load tests.

786 **6.2 Brackets:** Bracket capacities, P1, shall include reference to the type of bracket
787 and shall include provisions for segmented pipe section capacity, P2. The allowable
788 capacities of brackets connected to or embedded in concrete shall provide values for
789 systems installed in the different concrete strengths that were evaluated. Installation
790 shall be limited to uncracked concrete as defined in the applicable code. The table of
791 capacities for brackets and segmented pipe sections shall indicate whether the structure to
792 be supported has to be side-sway braced or rotationally fixed based on assumptions used

793 in the design and testing of the product.

794 **6.3 Segmented Pipe Sections:** Segmented pipe section capacities shall be tabulated
795 for each size of segmented pipe for the conditions of being braced or unbraced in soft
796 and firm soils, as applicable. The evaluation report shall define these conditions by
797 reference to Chapter 18 of the IBC. Standard penetration resistance blow count ranges
798 for firm and soft soils described in Section 3.11.2.1 of this criteria shall be repeated in
799 evaluation reports. The evaluation report shall state that the segmented pipe capacity of
800 belled segmented pipe foundations in fluid soils shall be determined by a registered
801 professional engineer. For evaluation reports including provisions for lateral resistance,
802 the structural properties of the segmented pipe shall be provided including gross area,
803 section modulus, modulus of elasticity, maximum allowable bending moment, and
804 maximum allowable shear.

805 **6.4 Bullet Assemblies:** Bullet assembly compression and tension capacities shall be
806 tabulated for each type available.

807 **6.5 Soil Capacity:** If a soils load capacity correlation was validated, it shall be listed in the
808 evaluation report. Otherwise, the evaluation report shall indicate that soil capacity in
809 compression or tension needs to be determined by a registered design professional.
810 For lateral soil resistance, the evaluation report shall contain a table of capacities for all
811 soil types used in the lateral load testing. The evaluation report shall state that lateral
812 soil resistance shall be determined by a registered design professional for soil
813 conditions that differ from those shown in the table.

814 **6.6 Materials:** The evaluation report shall list the material composition, including steel

815 grades, of system and device components. Minimum material specifications for structures
816 to be supported on brackets included in the evaluation report shall be included, as
817 applicable.

818 **6.7 Design:** The evaluation report shall describe general procedures for design and
819 application of the belled segmented pipe foundation system or device and state
820 whether bracket capacity is based on a braced or unbraced belled segmented pipe system
821 or device in accordance with IBC Section 1808. An explanation shall be provided of the
822 structural analysis that shall be performed by the design professional for proper
823 application of the system or device, including consideration of the internal shears and
824 moment due to structure eccentricity and maximum span between belled segmented
825 pipe foundations. The magnitude of shear and moment forces exerted on the
826 structure due to the connection of the structure to the belled segmented pipe foundation or
827 device, shall be provided. The results of this analysis and the structural capacities shall be
828 used to select segmented pipe foundation devices. The evaluation report shall indicate
829 that Section 1808 of the IBC shall apply to these products.

830 **6.8 Foundation and Soils Investigation Report:** The evaluation report shall indicate
831 that a site-specific foundation and soils investigation report is required for proper
832 application of these products. The foundation and soils investigation report shall
833 address corrosive properties of the soil to ensure that a potential pile corrosion situation
834 does not exist. The foundation and soils investigation report shall address the support
835 conditions for the segmented pipe section. The foundation and soils investigation report
836 shall address the axial compression, axial tension, and lateral load soil capacities if
837 values cannot be determined from the evaluation report. The foundation and soils

838 investigation report shall address effects of groundwater and other questionable
839 characteristics.

840 **6.9 Installation:** The evaluation report shall note any special training or certification
841 required for installation professionals, equipment required for installation, and a detailed
842 description of proper installation techniques. Requirements and procedures for
843 quality assurance inspection of product installation shall be described, including
844 procedures for field verification of ultimate maximum soil capacity for tension and
845 compression through correlations with final bullet assembly elevation, as applicable. The
846 evaluation report shall state that for tension applications, the total length of segmented pipe
847 sections shall be determined by a registered professional engineer.

848 **6.10 Special Inspection:** For installation, the evaluation report shall state that special
849 inspection in accordance with Section 1704.9 of the IBC or Section 1701 .5.11 of the UBC
850 is required. Where on-site welding is required, the evaluation report shall state that
851 special inspection in accordance with Section 1704.3 of the IBC or Section 1701 .5.5
852 of the UBC is required. The evaluation report shall state the items to be observed by the
853 special inspector. At a minimum, these items shall include verification of
854 manufacturer, segmented pipe sections, couplers, bullet assembly and bracket
855 configuration, the installation depth of the foundation, and compliance of the
856 installation of belled segmented pipe foundation system with the approved
857 construction documents and this evaluation report. In lieu of continuous special
858 inspection, periodic special inspection in accordance with IBC Section 1701 .6.2 may be
859 permitted when structural observations in accordance with IBC Section 1702, a periodic
860 inspection schedule (prepared by the registered design professional), and evidence of

861 installer training by the report holder are provided to the code official.

862 **6.11 Identification:** The evaluation report shall describe the identification method used
863 by the manufacturer as set forth in Section 2.1.4.

864 **6.12 Findings:** The evaluation report shall list approved manufacturing facilities and
865 their inspection agencies

TABLE 1—REFERENCE STANDARD EDITIONS

STANDARD	IBC	UBC
ANSI AF&PA NDS	2005	1991 revised
AISC ASD	AISC 360-05	June 1, 1989
AISC LRFD	AISC 360-05	March 16, 1991
AWS D1.1	2004	1992

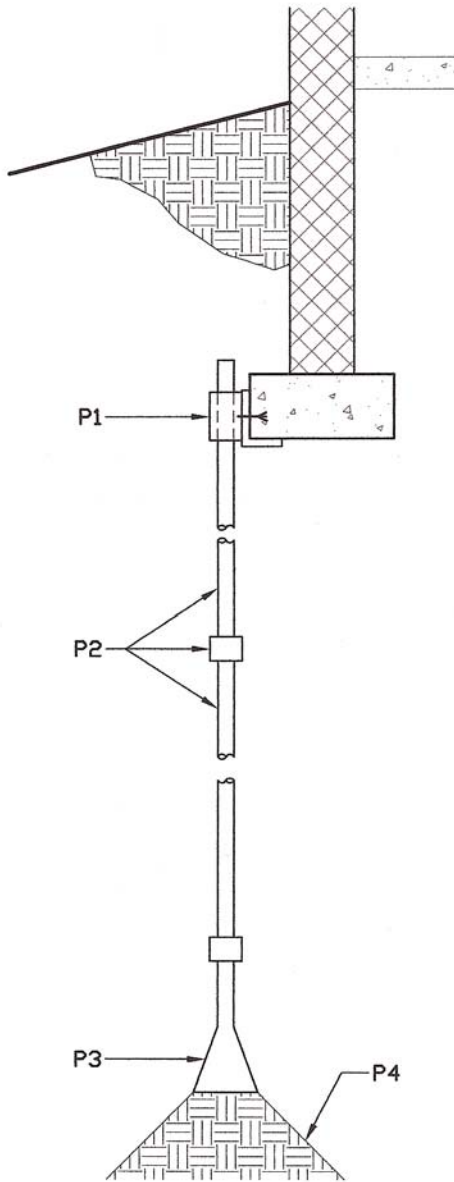


FIGURE 1—SEGMENTED PIPE FOUNDATION DEVICES

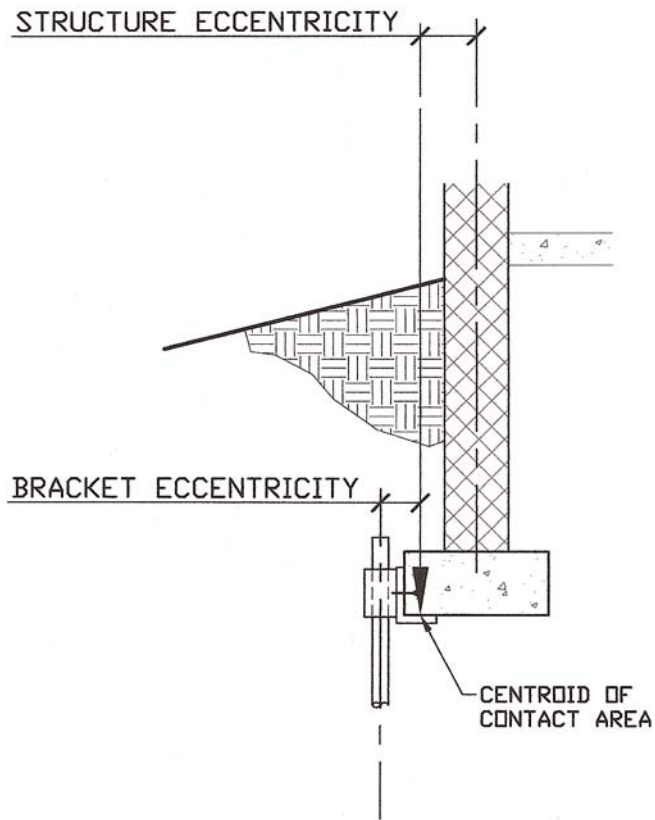


FIGURE 2—SIDE LOAD BRACKET FREE BODY DIAGRAM

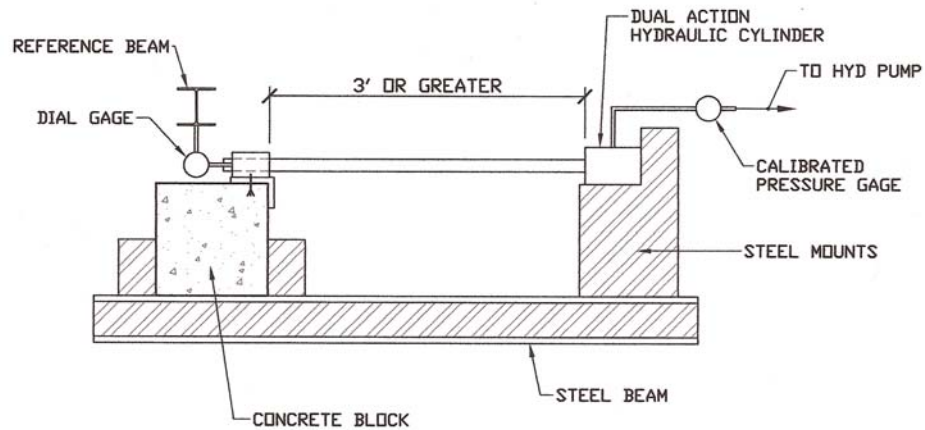


FIGURE 3—BRACKET EXAMPLE LABORATORY TEST SETUP

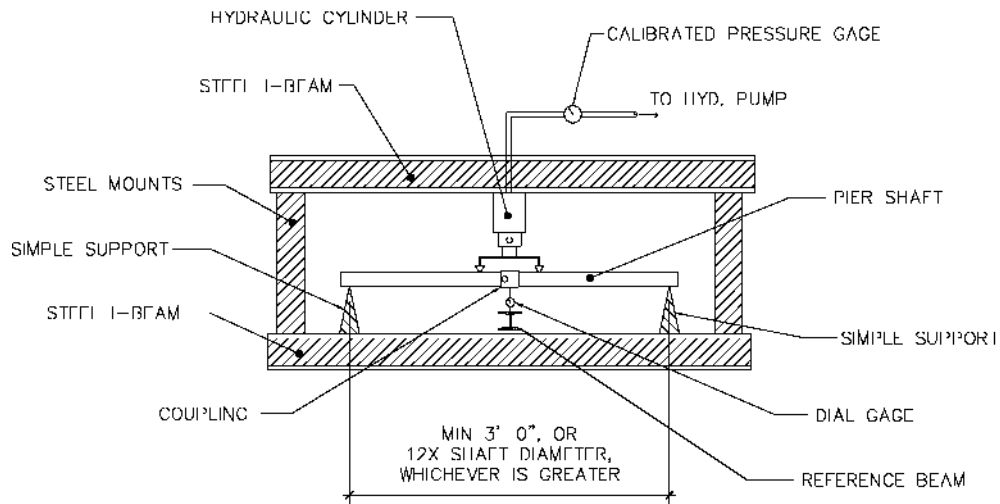


FIGURE 4—SEGMENTED PIPE SECTION BENDING EXAMPLE LABORATORY TEST SETUP

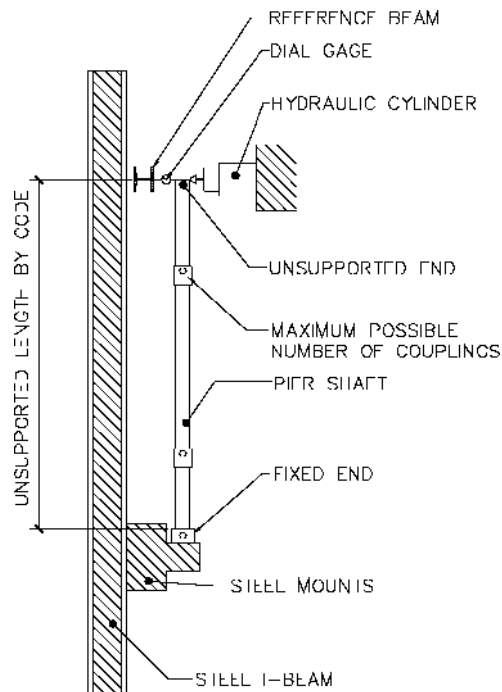


FIGURE 5—COUPLER RIGIDITY EXAMPLE LABORATORY TEST SETUP