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ESR-1396

Reissued 03/2017
This report is subject to renewal 03/2018.

DIVISION: 04 00 00—MASONRY
SECTION: 04 05 19.16—MASONRY ANCHORS

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

**5956 WEST LAS POSITAS BOULEVARD
PLEASANTON, CALIFORNIA 94588**

EVALUATION SUBJECT:

SIMPSON STRONG-TIE WEDGE-ALL® ANCHORS



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Section: 04 05 19.16—Masonry Anchors

REPORT HOLDER:

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(800) 999-5099
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EVALUATION SUBJECT:

SIMPSON STRONG-TIE WEDGE-ALL® ANCHORS

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2015, 2012, 2009 and 2006 *International Building Code*® (IBC)
- 2015, 2012, 2009 and 2006 *International Residential Code*® (IRC)

Property evaluated:

Structural

2.0 USES

Wedge-All® anchors are expansion anchors post-installed in predrilled holes in the face of fully grouted concrete-masonry (CMU) construction.

The Wedge-All anchors are alternatives to cast-in-place anchors described in Section 8.1.3 (2013 edition), or Section 2.1.4 (2011, 2008, 2005 editions) of TMS 402/ACI 530/ASCE 5 as referenced in Section 2107.1 of the IBC. The anchors are permitted to be used in structures regulated by the IRC, provided an engineered design is submitted in accordance with IRC Section [R301.1.3](#).

3.0 DESCRIPTION

3.1 Wedge-All® Anchors:

Wedge-All Anchors consist of a steel rod threaded at the upper end with standard coarse bolt threads and a machined or cold-formed tapered cone mandrel on the lower end. Attached to the cone mandrel is an expander ring. The diameter of the tapered mandrel increases toward the base of the anchor, permitting the expander ring to expand during anchor installation. The expander ring consists of a split-ring element with two dimples that engage the wall of the hole as it expands. A typical anchor is shown in [Figure 1](#).

3.2 Materials:

3.2.1 Wedge-All® Anchors: The Wedge-All® anchor is manufactured from carbon steel with a minimum yield strength of 50,000 psi (345 MPa) and a minimum tensile strength of 75,000 psi (517 MPa). The expander ring is formed from carbon steel with a minimum hardness of Rockwell B60. Steel parts are zinc-plated in accordance with [ASTM B633](#), Service Condition SC1, Type III; or are mechanically galvanized as a minimum in accordance with [ASTM B695](#), Class 55, Type I.

3.2.2 Fully Grouted CMU Construction: Fully grouted CMU construction must comply with [Chapter 21](#) of the IBC. The compressive strength of masonry, f_m , at 28 days must be a minimum of 1,500 psi (10.3 MPa). The concrete masonry must be fully grouted and constructed from the following materials:

3.2.2.1 Concrete Masonry Units (CMUs): CMUs must be minimum Grade N, Type II, lightweight, medium-weight, or normal-weight conforming to [ASTM C90](#). The minimum allowable nominal size of the CMU must be 8 inches (203 mm) wide by 8 inches (203 mm) high by 16 inches (406 mm) long (i.e., 8×8×16).

3.2.2.2 Grout: Grout must comply with Section 2103.3 of the 2015 IBC; Section [2103.13](#) of the 2012 IBC; Section [2103.12](#) of the 2009 and 2006 IBC; or Section R606 of the 2015 IRC; Section [R609.1.1](#) of the 2012, 2009 and 2006 IRC, as applicable. Alternatively, the grout must have a minimum compressive strength when tested in accordance with [ASTM C1019](#) equal to its specified strength, but not less than 2,000 psi (13.8 MPa).

3.2.2.3 Mortar: Mortar must be Type M or S in compliance with Section 2103 of the IBC, or Section R606.2.11 of the 2015 IRC; Section [R607.1](#) of the 2012, 2009 and 2006 IRC, as applicable.

4.0 DESIGN AND INSTALLATION

4.1 Design:

4.1.1 General: Wedge-All anchors described in this report must be assigned allowable tension and shear loads for designs based on allowable stress design (working stress design).

4.1.2 Design of Wedge-All (Carbon-steel) Anchors Installed in Fully Grouted CMU Construction: For installations in the face of fully grouted uncracked concrete masonry, the Wedge-All (carbon-steel) anchors are recognized to resist dead-, live-, wind-, and earthquake-load applications.

Allowable tension and shear loads, critical and minimum edge and end distances, and critical spacing requirements for anchors installed in the face of concrete masonry construction are noted in [Table 2](#) for applications regulated by the IBC or IRC. Allowable load reduction factors for anchors installed at distances less than critical edge distance are noted in [Table 3](#) for IBC and IRC applications. Refer to [Figure 2](#) for allowable anchor location and critical and minimum edge distances.

Allowable loads for the Wedge-All (carbon-steel) anchors installed in the face of fully grouted concrete masonry subjected to combined shear and tension forces must be determined by the following equation:

$$\left(\frac{P_s}{P_t}\right)^{5/3} + \left(\frac{V_s}{V_t}\right)^{5/3} \leq 1.0$$

where:

- P_s = Applied service tension load.
- P_t = Allowable service tension load.
- V_s = Applied service shear load.
- V_t = Allowable service shear load.

4.2 Installation:

4.2.1 General: The Wedge-All[®] anchor must be installed in a predrilled hole in the base material (i.e., concrete masonry construction) of the same diameter as the nominal diameter of the anchor. Holes must be drilled with a bit complying with [ANSI B212.15-1994](#) and must be drilled to a depth allowing proper embedment. The anchor must be inserted to a minimum of six threads below the substrate surface, and the nut must be installed on the anchor top and tightened on the washer bearing on the substrate until the torque indicated in [Table 2](#) is attained. The depth of the drilled holes must exceed the anchor embedment length by a sufficient amount to permit the anchors to be installed to a minimum of six threads below the substrate surface.

4.2.2 Installation in CMU Construction: The allowable tension and shear loads are for the Wedge-All (carbon-steel) anchors installed in the fully grouted cells, the center web of concrete masonry units (CMU), and horizontal mortared bed joints of fully grouted concrete masonry construction. Allowable loads for anchors installed within 1¹/₄ inches (31.7 mm) of the vertical head joint between CMU of the concrete masonry construction are outside the scope of this report. Refer to [Figure 2](#) for details.

4.3 Special Inspection (When Required):

Special inspection must be provided when required by the tables in this evaluation report. Special inspection under the IBC and IRC must conform to IBC Sections [1704](#) and [1705](#).

For fasteners installed with special inspection, the following items, as applicable, must be inspected: fastener type and dimensions; masonry unit type and compliance with ASTM C90; grout and mortar compressive strengths, and (when required) masonry prism compressive strength; drill bit size and compliance with ANSI B212.15-1994; and fastener embedment, spacing, and edge (and end) distances. The special inspector must inspect and verify that anchor installation complies with this evaluation report and Simpson Strong-Tie Company's published installation instructions.

5.0 CONDITIONS OF USE

The Wedge-All[®] anchor described in this report complies with, or is a suitable alternative to what is specified in,

those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1** Anchors are identified and installed in accordance with this report and the manufacturer's published installation instructions.
- 5.2** Design of Wedge-All[®] (carbon-steel) anchors installed in the face of fully grouted concrete masonry construction to resist dead, live, wind and earthquake load applications must be in accordance with Section 4.1 of this report.
- 5.3** For installations in CMU construction, anchors are recognized to resist static, earthquake, and wind loads. When using the basic load combinations in accordance with IBC Section [1605.3.1](#), allowable loads are not permitted to be increased for seismic or wind loading. When using the alternative basic load combinations from the 2009 and 2006 IBC Section [1605.3.2](#) that include wind or earthquake loads, the allowable tension and shear loads shown in [Table 2](#) for Wedge-All[®] (carbon-steel) anchors installed in the face of fully grouted concrete masonry may be increased by 33¹/₃ percent. Alternatively, the load combinations in IBC Section [1605.3.2](#) may be multiplied by a factor of 0.75. For the 2015 and 2012 IBC, the allowable loads or load combinations are not permitted to be adjusted. Refer to [Table 1](#) of this report.
- 5.4** Anchors must be installed in accordance with Section 4.0, and the holes for the anchors must be predrilled with carbide-tipped masonry drill bits complying with ANSI B212.15-1994 into the approved substrate, and have the same diameter as the nominal diameter of the anchor.
- 5.5** Since an ICC-ES acceptance criteria for evaluating data to determine the performance of anchors subjected to fatigue or shock loading is unavailable at this time, the use of these anchors under these conditions is outside the scope of this report.
- 5.6 Fire-resistive Construction:** Where not otherwise prohibited in the applicable code, anchors are permitted for use with fire-resistance-rated construction provided that at least one of the following conditions is fulfilled:
 - Anchors are used to resist wind or seismic forces only.
 - Anchors that support fire-resistance-rated construction or gravity load-bearing structural elements are within a fire-resistance-rated envelope or a fire-resistance-rated membrane, are protected by approved fire-resistance-rated materials, or have been evaluated for resistance to fire exposure in accordance with recognized standards.
 - Anchors are used to support nonstructural elements.
- 5.7** Since an ICC-ES acceptance criteria for evaluating the performance of expansion anchors in cracked masonry is unavailable at this time, the use of anchors is limited to installation in uncracked masonry. Cracking occurs when $f_t > f_r$ due to service loads or deformations.
- 5.8** Calculations demonstrating that the applied loads are less than the allowable loads described in this report must be submitted to the code official at the time of

permit application. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

- 5.9 Special inspection, when required, must be provided in accordance with Section 4.3.
- 5.10 Use of zinc-plated anchors must be limited to dry, interior locations. Use of mechanically galvanized (according to ASTM B695), Class 65, Type I anchors is permitted in exterior-exposure or damp environments.
- 5.11 Wedge-All anchors are manufactured by Simpson Strong-Tie under a quality control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Expansion Anchors in Masonry Elements (AC01), dated November 2015.

7.0 IDENTIFICATION

The Simpson Strong-Tie Company, Inc., Wedge-All® anchors are identified in the field by dimensional characteristics and packaging. The packaging label notes the name and address of Simpson Strong-Tie Company, Inc.; the manufacturing location; the anchor type, size, and length; and the ICC-ES evaluation report number (ESR-1396). The threaded end of each Wedge-All anchor is stamped with a length identification code letter as indicated in [Table 4](#).



FIGURE 1—WEDGE-ALL® ANCHOR

TABLE 1—PERCENT INCREASE FOR WIND AND EARTHQUAKE LOADING CONDITIONS

ANCHOR (Material Type)	SUBSTRATE	PERCENT INCREASE FOR SHORT-TERM LOADING CONDITIONS	
		Tension	Shear
Wedge-All (carbon steel)	Fully Grouted CMU Masonry ^(1,2)	33 ¹ / ₃	33 ¹ / ₃

¹When using the basic load combinations in accordance with IBC Section [1605.3.1](#), allowable loads must not be increased for wind or earthquake loading.

²When using the alternative basic load combinations in 2009 or 2006 IBC Section [1605.3.2](#) that include wind or earthquake loads, the allowable shear and tension loads for anchors may be increased by the tabulated percentage increases. Alternatively, the alternate basic load combinations may be reduced by multiplying them by 0.75 when using 2009 or 2006 IBC Section 1605.3.2, as applicable. For the 2015 or 2012 IBC, the allowable loads or load combinations are not permitted to be adjusted.

TABLE 2—IBC AND IRC ALLOWABLE TENSION AND SHEAR LOADS FOR THE WEDGE-ALL® ANCHORS INSTALLED IN THE FACE OF FULLY GROUTED CMU MASONRY CONSTRUCTION¹

ANCHOR DIAMETER (in.)	EMBEDMENT DEPTH ² (in.)	INSTALLATION TORQUE (ft.-lbs.)	ANCHOR LOCATION ^{3,4} (inches)			ALLOWABLE LOADS FOR ANCHORS INSTALLED AT CRITICAL EDGE DISTANCE, C_{crit} , AND CRITICAL SPACING, S_{crit} (lbf)	
			Edge Distance		Critical Spacing, S_{crit}	Tension ⁵	Shear ⁵
			Critical, C_{crit}	Minimum, C_{min}			
3/8	2 5/8	30	10 1/2	4	10 1/2	340	670
1/2	3 1/2	35	14	4	14	425	1,070
5/8	4 3/8	55	17 1/2	4	17 1/2	625	1,635
3/4	5 1/4	120	21	4	21	865	2,035

For SI: 1 inch = 25.4 mm, 1 psi = 6.89 kPa, 1 lbf = 4.48 N.

¹Allowable loads are based on special inspection being provided during anchor installation. Special inspection requirements must comply with Section 4.3 of this report.

²Embedment depth is measured from the outside face of the masonry.

³Critical and minimum edge distances, C_{crit} and C_{min} , respectively, and critical spacing, S_{crit} , must comply with this table. Refer to Figure 2. Critical edge distance and critical spacing are valid for anchors resisting the tabulated allowable tension or shear loads. Refer to [Table 3](#) for allowable tension and shear load reduction factors for anchors installed between critical and minimum edge distances.

⁴Anchors must be installed a minimum of 1 1/4 inches from vertical head joints and T-joints. Refer to Figure 2 for permitted and prohibited anchor installation locations. Refer to Section 4.2 of this report for installation details.

⁵Tabulated allowable loads are based on a factor of safety of 5.

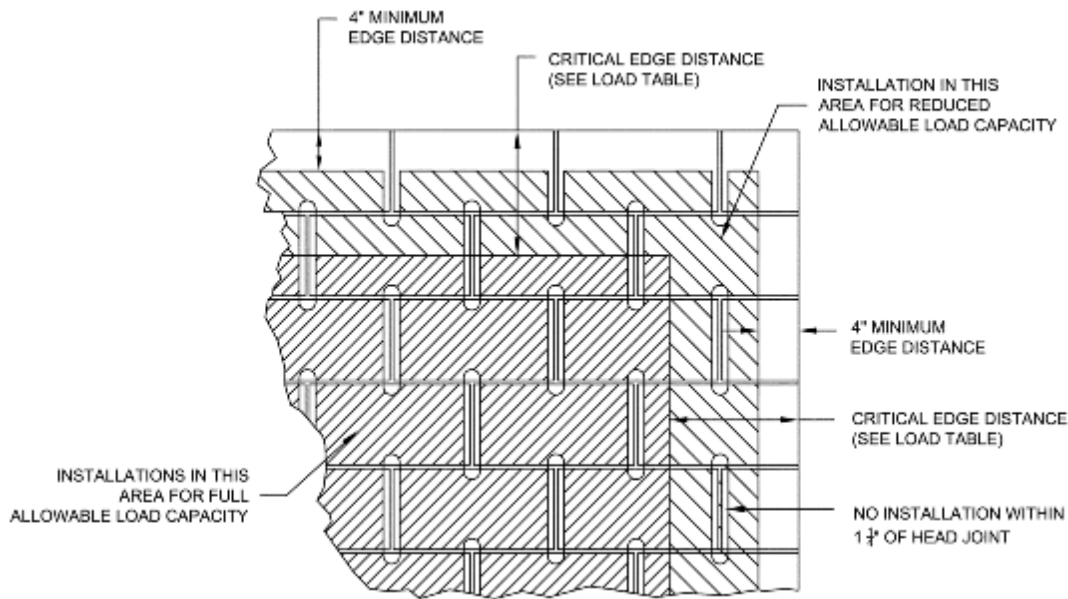


FIGURE 2—WEDGE-ALL® ANCHOR INSTALLED IN THE FACE OF FULLY GROUTED CMU MASONRY CONSTRUCTION (Refer to Tables 2 and 3)

TABLE 3—LOAD REDUCTION FACTORS FOR WEDGE-ALL® ANCHORS INSTALLED BETWEEN CRITICAL AND MINIMUM EDGE DISTANCES (ANCHORS INSTALLED IN THE FACE OF FULLY GROUTED CMU MASONRY)^{1,2,3}

ANCHOR DIAMETER (inch)	MINIMUM EMBEDMENT DEPTH, h_v (inches)	LOAD REDUCTION FACTORS FOR ANCHORS INSTALLED AT:		
		Critical Edge Distance, c_{crit}	Minimum Edge Distance, c_{min}	
		Tension or Shear Load	Tension Load	Shear Load
$\frac{3}{8}$	$2\frac{5}{8}$	1.0	1.0	0.79
$\frac{1}{2}$	$3\frac{1}{2}$	1.0	1.0	0.52
$\frac{5}{8}$	$4\frac{3}{8}$	1.0	0.8	0.32
$\frac{3}{4}$	$5\frac{1}{4}$	1.0	0.8	0.32

For SI: 1 inch = 25.4 mm.

¹The load reduction factors in this table are applicable to the allowable loads shown in [Table 2](#).

²Reduction factors are cumulative. Multiple reduction factors for more than one spacing or edge distance are calculated separately and multiplied.

³Load reduction factors for anchors loaded in tension or shear with edge distances between critical and minimum are obtained by linear interpolation.

TABLE 4—ANCHOR LENGTH IDENTIFICATION CODE

CODE ¹		LENGTH OF ANCHOR	
		inches	mm
A	Black	$1\frac{1}{2} < 2$	38 < 51
B	White	$2 < 2\frac{1}{2}$	51 < 63
C	Red	$2\frac{1}{2} < 3$	63 < 76
D	Green	$3 < 3\frac{1}{2}$	76 < 89
E	Yellow	$3\frac{1}{2} < 4$	89 < 102
F	Blue	$4 < 4\frac{1}{2}$	102 < 114
G	Purple	$4\frac{1}{2} < 5$	114 < 127
H	Brown	$5 < 5\frac{1}{2}$	127 < 140
I	Orange	$5\frac{1}{2} < 6$	140 < 152
J	N/A	$6 < 6\frac{1}{2}$	152 < 165
K	N/A	$6\frac{1}{2} < 7$	165 < 178
L	N/A	$7 < 7\frac{1}{2}$	178 < 191
M	N/A	$7\frac{1}{2} < 8$	191 < 203
N	N/A	$8 < 8\frac{1}{2}$	203 < 216
O	N/A	$8\frac{1}{2} < 9$	216 < 229
P	N/A	$9 < 9\frac{1}{2}$	229 < 241
Q	N/A	$9\frac{1}{2} < 10$	241 < 254
R	N/A	$10 < 11$	254 < 267
S	N/A	$11 < 12$	267 < 305
T	N/A	$12 < 13$	305 < 330
U	N/A	$13 < 14$	330 < 366
V	N/A	$14 < 15$	366 < 381
W	N/A	$15 < 16$	381 < 406
X	N/A	$16 < 17$	406 < 432
Y	N/A	$17 < 18$	432 < 457
Z	N/A	$18 < 19$	457 < 483

¹N/A = Not applicable